

Guidelines for Certification

Part 10 - Industry

Volume 2

GUIDELINES FOR OFFSHORE CONCRETE STRUCTURES

2020

Biro Klasifikasi Indonesia



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Foreword

Annex K

This Guidelines provides principles, technical requirements for the design, construction and in service inspection of Offshore Concrete Structures. The general description about this guidelines in every section describe below:

Section 1	General
Section 2	Safety philosophy
Section 3	Design documentation
Section 4	Materials
Section 5	Loads and analyses requirements
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Annex G	QA/QC system for manufacture of FRP bars
Annex H	Requirements to content in certificate for structural grout
Annex I	QA/QC system for manufacture of structural grout
Annex J	Mock-up test requirements

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Supplemental requirements for steel reinforcement specified for use with this guidelines

Further queries or comments concerning this Guidelines are welcomed through communication to BKI Head Office.

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Section 1 General

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A. Introduction

- 1.1 This Guideline shall be used together with the general offshore design standards for steel structures Rules for Structures (Pt.5, Vol.II) or other equivalent standards recognized by BKI.
- **1.2** For design and construction of offshore concrete support structures for wind turbines, see Rules for Fixed Offshore Installation (Pt.5, Vol.VII) or other equivalent standards recognized by BKI.
- **1.3** For design and construction of LNG terminal structures and containment systems, see Guidelines for Floating Offshore Liquefied Gas Terminals (Pt.5, Vol.2).

B. Objective

The objectives of this Guideline are to:

- Provide a standard for the design, fabrication/construction, installation and in-service inspection of
 offshore concrete structures with an acceptable level of safety by defining minimum requirements
 for design, construction control and in-service inspection.
- Provide technical requirements for certification of cementitious structural grouts and FRP reinforcement.
- Serve as a contractual reference document between supplier and purchasers related to design, construction and in-service inspection of offshore concrete structures.
- Serve as a guideline for designer, supplier, purchasers and regulators.

C. Scope

- 1. The guideline comprises of eight (8) sections focusing on:
 - safety philosophy
 - design documentation
 - materials
 - loads and analysis requirements
 - detailed design of offshore concrete structures
 - construction
 - in-service inspection,
 - maintenance and condition monitoring.

Sec 1 General C-D

2. In addition, several annexes give more detailed information. Informative annexes are non-mandatory.

- Annex A contains guidelines for evaluation of environmental loading.
- Annex B and Annex C contain guidelines for modelling and structural analysis.
- Annex D contains guidelines for the use of alternative design standards.
- Annex E contains guidelines for calculation of crack widths.
- Annex F specifies requirements for fibre reinforced polymer (FRP) bars subject to BKI certification service
- Annex G contains minimum requirements for the QA/QC system for manufacture of FRP bars.
- Annex H specifies requirements for cementitious grouts subject to BKI certification services.
- Annex I contains minimum requirements for the QA/QC system for manufacture of structural grout and fibre
- Annex J contains minimum requirements for mock-up testing for cementitious grouts subject to BKI certification services.
- Annex K contains minimum requirements for steel reinforcement specified for use with this guideline.
- **3.** This Guideline specifies:
 - Principles, technical requirements and guidelines for the design, fabrication/construction, installation and in-service inspection of offshore concrete structures.
 - Minimum technical requirements for certification of cementitious structural grouts and FRP reinforcement.
- 4. This guideline covers fixed and floating structures where reinforced concrete, prestressed concrete and cementitious grout are used as structural materials.
- 5. In addition to the requirements provided in this guideline, it is the responsibility of the designer, owner and operator to comply with additional requirements that may be imposed by the flag state or any other jurisdictions in the intended area of deployment and operation.

D. Application

1. General

- **1.1** The Guidelines may be used in the design, fabrication/construction, installation and in-service inspection of the following types of support structures, which are referred in this guideline as offshore concrete structures:
 - gravity based structures (GBS) for oil/gas production offshore
 - GBS for oil/gas production with oil/gas storage facility
 - floating concrete structures for production/storage of oil/gas. The structure may be of any type:
 floating
 - structure, e.g. tension leg platform (TLP), column stabilized units
 - barge units
 - deep-water caisson type concrete foundation of bridges
 - floating foundations for bridges, parking houses or storage buildings
 - other types of offshore/nearshore concrete structures.

Sec 1 General D-E

1.2 The development and design of new concepts for offshore concrete structures requires a systematic hazard identification process in order to mitigate the risk to an acceptable risk level. Hazard identification is therefore a central tool in this guideline.

E. References

1. General

In this guideline, when dated references of BKI guideline are presented, only the edition cited applies. For undated references, the latest edition of the referenced document (including amendments) applies.

2. Standards other than BKI guideline

- 2.1 In case of conflict between the requirements of BKI guideline and a reference document other than BKI guideline, the requirement of BKI guideline shall prevail.
- 2.2 The provision for using standards other than BKI guideline is that the same safety level as provided by this BKI guideline is obtained.
- 2.3 Where reference is made to standards other than BKI guideline, the valid revision shall be taken as the revision which is current at the date of issue of this guideline, unless otherwise noted.

References

- 3.1 The standards in Table 1.1 include provisions, which through reference in this text constitute provisions and acceptable methods for fulfilling the requirements of this guideline.
- 3.2 Other recognised standards may be applied provided it is documented that they meet or exceed the level of safety of this BKI guidelines, see Annex D.

Reference	Title	
Part 5, Vol II	Rules for structures	
Part 5, Vol VII	Rules for Fixed Offshore Installation	
Part 5, Vol 2	Guidelines for Floating Offshore Liquefied Gas Terminal	
Part 1, Vol. IX	Rules for Ships Carrying Liquefied gases in bulk	
Part 1, Vol. V	Rules for Materials	
Part 5, Vol. IX	Rules for single point mooring	
Part 5, Vol. C	Guidance for buckling and ultimate strength	
Part 5, Vol. B	Guidance for Fatigue assessment	

Table 1.1 BKI References

Table 1.2 Other references

Reference	Title	
ACI 440.1R-15	Guide for the Design and Construction of Structural Concrete Reinforced with Fibre	
	Reinforced Polymer (FRP) Bars	
ACI 440.3R-12	Guide Test Methods for Fibre-Reinforced Polymer (FRP) Composites for Reinforcing or	
	Strengthening Concrete and Masonry Structures	
ACI 440-4R-04	Prestressing Concrete Structures with FRP Tendons	
ACI 440R-07	Report on fibre-reinforced polymer (FRP) reinforcement for concrete structures.	
ASTM C114	Standard Test Method for Chemical Analysis of Hydraulic Cement	
ASTM C150	Standard Specification for Portland Cement	
ASTM C157	Standard Test Method for Length Change of Hardened Hydraulic-Cement Mortar and	
	Concrete	

Table 1.2 Other references (Continued)

Reference	Title
ASTM C187	Standard Test Method for Amount of Water Required for Normal Consistency of
	Hydraulic Cement Paste
ASTM C191	Standard Test method for Time of Setting of Hydraulic Cement by Vicat Needle
ASTM C204	Standard Test Method for Fineness of Hydraulic Cement by Air-Permeability Apparatus
ASTM C230	Standard Specification for Flow Table for Use in Tests of Hydraulic Cement
ASTM C348	Standard Test Method for Flexural Strength of Hydraulic Cement Mortars
ASTM C403	Standard Test Method for Time of Setting of Concrete Mixtures by Penetration
	Resistance
ASTM C457	Standard Test Method for Microscopical Determination of Parameters of the Air-Void
	System in Hardened Concrete
ASTM C469	Standard Test Method for Static Modulus of Elasticity and Poisson's Ratio of Concrete
	in Compression
ASTM C490	Standard Practice for Use of Apparatus for the Determination of Length Change of
	Hardened Cement Paste, Mortar, and Concrete
ASTM C496	Standard Test Method for Splitting Tensile strength of Cylindrical Concrete Specimens
ASTM C512	Standard Test Method for Creep of Concrete in Compression
ASTM C666	Standard Test Method for Resistance of Concrete to Rapid Freezing and Thawing
ASTM C940	Standard Test Method for Expansion and Bleeding of Freshly Mixed Grouts for
	Preplaced Aggregate Concrete in the Laboratory
ASTM C1437	Standard Test Method for Flow of Hydraulic Cement Mortar
ASTM C1581	Standard Test Method for Determining Age at Cracking and Induced Tensile Stress
	Characteristics of Mortar and Concrete under Restrained Shrinkage
ASTM C1698	Standard Test Method for Autogenous Strain of Cement Paste and Mortar
CSA S806-12	Design and construction of building structures with fibre-reinforced polymers
CSA A23.1-14	Concrete materials and method of concrete construction
CSA A23.2-14	Test methods and standard practices for concrete
EN 196-1	Methods of testing cement – Part 1: Determination of strength
EN 196-2	Methods of testing cement – Part 2: Chemical analysis of cement
EN 196-3	Methods of testing cement – Part 3: Determination of setting times and soundness
EN 196-6	Methods of testing cement – Part 6: Determination of fineness
EN 206	Concrete - Specification, performance, production and conformity
EN 445	Grout for prestressing tendons - Test methods
EN 447	Grout for prestressing tendons - Basic requirements
EN 1015-6	Methods of test for mortar for masonry - Part 6: Determination of bulk density of fresh
	mortar
EN 1015-7	Methods of test for mortar for masonry - Part 7: Determination of air content of fresh
EN 40050 C	mortar
EN 12350-6	Testing fresh concrete - Part 6: Density
EN 12350-7	Testing fresh concrete - Part 7: Air content - pressure methods
EN 12350-8	Testing fresh concrete - Part 8: Self-compacting concrete - Slump flow test
EN 12390-1	Testing hardened concrete - Part 1: Shape, dimensions and other requirements for
EN 12200 2	specimens and moulds
EN 12390-2	Testing hardened concrete - Part 2: Making and curing specimens for strength tests
EN 12390-3	Testing hardened concrete - Part 3: Compressive strength of test specimens
EN 12390-6	Testing hardened concrete - Part 6: Tensile splitting strength of test specimens
EN 12390-7	Testing hardened concrete – Part 7: Density of hardened concrete
EN 12504-1	Testing concrete in structures - Part 1: Cored specimens - Taking, examining and
EN 12670	testing in compression Execution of concrete structures
EN 13670	
EN 13791	Assessment of in-situ compressive strength in structures and precast concrete
ISO 1020 4	components Testing of concrete Part 4: Strength of hardened concrete
ISO 1920-4	Testing of concrete - Part 4: Strength of hardened concrete Fibra reinforced polymer (FRR) reinforcement of concrete Test methods - Part 1 FRR
ISO 10406-1	Fibre-reinforced polymer (FRP) reinforcement of concrete – Test methods – Part 1 FRP bars and grids
	Dai 3 and grius

Sec 1 General E-F

Table 1.2 Other references (Continued)

Reference	Title	
ISO 19900	Petroleum and natural gas industries – General requirements for offshore	
	structures	
ISO 19901-1	Petroleum and natural gas industries – Specific requirements for offshore	
	structures – Part 1: Metocean design and operating considerations	
ISO 19901-2	Petroleum and natural gas industries – Specific requirements for offshore	
	structures – Part 2: Seismic design procedures and criteria	
ISO 19901-4	Petroleum and natural gas industries – Specific requirements for offshore	
	structures – Part 4: Geotechnical and foundation design considerations	
ISO 19901-8	Petroleum and natural gas industries – Specific requirements for offshore	
	structures – Part 8: Marine soil investigations	
ISO 19903	Petroleum and natural gas industries – Fixed concrete offshore structures	
MC2010	fib Model Code for Concrete Structures 2010	
NORSOK	N-003 Actions and Actions Effects	
NORSOK	N-004 Design of Steel Structures	
SINTEF STF22	Eurocrete. Modifications to NS3473 when using fibre reinforced plastic (FRP)	
A98741	reinforcement	

F. Definitions

1. Terms

Terms and definitions as shown in Table 1.3 are used in this guideline.

Table 1.3 Definitions of terms

Terms	Definitions
abnormal level	intense earthquake of abnormal severity under the action of which the structure
earthquake (ALE)	should not suffer complete loss of integrity
accidental limit states	limit state related to the possibility of the structure to resist accidental loads and
(ALS)	maintain integrity and performance of the structure due to local damage
	or flooding
accidental loads (A)	rare occurrences of abnormal environmental loads, fire, flooding, explosions, dropped
aggregates	objects, collisions, unintended pressure differences, thermal shock due to LNG spilling
	or overflow, etc. constituent material of concrete or grout added to increase volume,
	weight or durability of the material Aggregates are the main constituent, both with
	respect to volume and weight, in a structural concrete mix. They may generally be
	divided into two groups, these being: sand or fine aggregate
	(materials less than 5 mm) and coarse aggregate (materials larger than 5 mm).
air gap	free distance between the design wave and the underside of a topside structure
	supported on columns allowing the wave to pass under the topside structure
as-built documentation	documentation of the offshore structure as finally constructed
	Section 3, A.5 presents the list of documents that are part of the as-built
	documentation.
atmospheric	the external surfaces of the unit above the splash zone
zone	
cathodic protection	technique to prevent corrosion of a steel surface by making the surface to be the
	cathode of an electrochemical cell
cement	binder component in a structural concrete or grout mix
cement grout	general term referring to grout batched at the work site consisting of mainly cement
	and water May refer to neat cement grout or a cement and water mix with a limited
	dosage of admixtures added during batching.

Sec 1 General F

Table 1.3 Definitions of terms (Continued)

Terms	Definitions
certification	third-party issue of a statement, based on a decision following review, that
	fulfilment of specified requirements has been demonstrated related to products,
	processes or systems Review shall in this context mean verification of the suitability,
	adequacy and effectiveness of selection and determination activities, and the
	results of these activities, with regard to fulfilment of specified requirements by an
	object of conformity assessment.(ISO 17000:2004).
certification service	the process of performing certification in accordance with a service specification
	and with a BKI certificate as deliverable.
characteristic load	reference value of a load to be used in the determination of load effects The
	characteristic load is normally based upon a defined fractile in the upper end of the
	distribution function for load.
characteristic material	nominal value of material strength to be used in the determination of the design
strength	resistance The characteristic material strength is normally based upon a 5% fractile
	in the lower end of the distribution function for material strength.
characteristic value	representative value associated with a prescribed probability of not being
	unfavourably exceeded during some reference period
classification	a service which comprises the development and maintenance of rules, and the
	verification of compliance with the rules throughout the Vessels' life.
	The extent of and methods for verifying compliance will be decided by the Society
	to establish reasonable assurance that the relevant rules are complied with.
coating	metallic, inorganic or organic material applied to steel surfaces for prevention of
	corrosion
concrete grade	parameter used to define the concrete strength
corrosion allowance	extra wall thickness added during design to compensate for any anticipated
	reduction in thickness during the operation
cryogenic temperature	being or related to very low temperature down to -200°C
deck mating	operations through which the deck floated on barges is mated with the concrete
d - f ti d - /D)	support structure
deformation loads (D)	loads effects on the structure caused by thermal effects, prestressing effects, creep/shrinkage effects, differential settlements/deformations, etc
design basis	document where owners' requirements in excess of this guideline should be given
design hazards	hazards likely to occur are identified as part of the risk assessment Design hazards
uesigii ilazarus	are mitigated into the structural design of the structure.
design life	duration to which the parameters used in structural design are related
design temperature	lowest daily mean temperature in air for areas where the unit will be transported,
acsign temperature	installed and operated for the given period of operation Temperature experienced
	by the element due to local effects during its design life, may include influences
	from cargo temperature, sea temperature, operational requirements etc
design value	value used in the deterministic design procedure, i.e. characteristic value modified
	by the resistance factor or load factor
driving voltage	he difference between closed circuit anode potential and the protection potential
ductility	property of a steel or concrete member to sustain large deformations without
	failure
environmental loads (E)	loads from wind, wave, tide, current, snow, ice and earthquake
expected loads and	history for a specified time period, taking into account the number of load cycles,
response history	the resulting load levels and response for each cycle
expected value	most probable value of a load during a specified time period
extreme level	earthquake with a severity which the structure should sustain without major
earthquake (ELE)	damage
	When exposed to an ELE, a structure is supposed to retain its full capacity for all
· · ·	subsequent conditions.
fatigue	degradation of the material caused by cyclic loading
fatigue critical	structure with calculated fatigue life near the design fatigue life
fatigue limit states (FLS)	limit state related to the possibility of failure due to the effect of cyclic loading

Sec 1 General F

Table 1.3 Definitions of terms (Continued)

Terms	Definitions
fibre mass fraction	ratio of fibre mass to total mass of FRP material within a reinforcement bar
fibre	short fibres made from steel or FRP used in structural concrete or grout
fluid	a liquid that in most cases considered will be seawater, water or oil
	A concrete structure might be exposed to the pressure and chemical properties of
	such fluids on the outer or inner face.
FRP material	fibre reinforced polymer (FRP) composite made from carbon, glass, aramid or basalt
fibre reinforced	structural concrete mixed with short fibre material
concrete	
fibre reinforced grout	grout mixed with short fibre material
fibre volume	ratio of fibre volume to total volume of FRP material within a reinforcement bar
fraction	
functional loads	permanent (G) and variable loads (Q), except environmental loads (E), to which the structure is exposed
grout	see cement grout, pre-blended grout, fibre reinforced grout and structural grout
hazards	List of critical situations that will have the potential to cause, or contribute
identification	substantially to a major accident if they happen to fail The list is based on
	consequence of failure only, not on likelihood of failure of the individual hazards.
headed	headed reinforcement bars are ordinary reinforcement bars with circular, square or
reinforcement	rectangular shaped steel plates attached at one or both ends, generally by means of
(T-heads)	friction welding
high strength	concrete of grade in excess of C65
concrete	
hindcasting	method using registered meteorological data to reproduce environmental
	parameters, which is mostly used for reproducing wave parameters
inspection	activities such as measuring, examination, testing, gauging one or more characteristics
	of an object or service and comparing the results with specified requirements to
	determine conformity
live loads of	live loads that the structure may be exposed to for its entire service life or a
permanent	considerable part of it, e.g. weight of furniture, stored goods etc
character	
live loads of	live loads that the structure may be exposed to only for limited durations much less
variable	than the service life, such as e.g. weight of occupants and (not permanently stored)
character	vehicles
light weight	aggregates made from expanded clay, expanded shale, slate or sintered pulverized
aggregate	ash from coal-fired power plants, or from other materials with corresponding
(LWA)	documented properties in accordance with the requirements of recognized standards
	e.g. ASTM, ACI, EN.
light weight	concrete made with lightweight aggregates conforming to requirements contained in
aggregate	recognized standards, e.g. relevant ASTM, ACI or EN standard
concrete (LWAC)	The provisions of this guideline are valid only for LWAC having a lower limit of oven-
	dry density of 1200 kg/m³ and an upper limit of 2200 kg/m³.
limit state	state beyond which the structure no longer satisfies the performance requirements.
	The following categories of limit states are of relevance for structures:
	ULS = ultimate limit states
	FLS = fatigue limit states
	ALS = accidental limit states
limata papa and a di	SLS = serviceability limit states
limit state design	design of the offshore concrete structure in the limit states of ULS, SLS, FLS and ALS
load and resistance	method for design where uncertainties in loads are represented with a load factor and
factor design (LRFD)	uncertainties in resistance are represented with a material factor
load effect	effect of a single design load or combination of loads on the equipment or system,
	such as stress, strain, deformation, displacement, motion, etc.

Table 1.3 Definitions of terms (Continued)

Terms	Definitions
lowest daily mean	lowest value on the annual mean daily average temperature curve for the area in
temperature	question,. for temporary phases or restricted operations the lowest daily mean
	temperature may be defined for specific seasons
	Mean daily average temperature = the statistical mean average temperature for a
	specific calendar day.
	— Mean: statistical mean based on number of years of observations.
	— Average: average during one day and night.
lowest waterline	typical light ballast waterline for ships, transit waterline or inspection waterline for
	other type of units
mill certificate	document made by the manufacturer of cement which contains the results of all the
	required tests and which certifies that the tests have been carried out by the
	manufacturer on sample taken from the delivered cement itself
neat cement grout	grout made from a mixture of cement and water only
non-cementitious	materials such as epoxy and polyurethane which are specially made for use together
materials	with structural concrete to improve the concrete properties or to supplement, repair
non dostructivo	or replace the concrete.
non-destructive	testing techniques used to evaluate the properties of materials, components or systems without causing damage such as rebound hammer, resistivity, radiographic,
testing	ultrasonic, impact eco for concrete testing and radiography, ultrasonic or magnetic
(NDT)	powder methods for inspection of welds
normal strength	concrete of grade C35 to C65
concrete	Concrete of grade C33 to C03
normal weight	ordinary concrete with an oven-dry density of at least 2200 kg/m³ and an upper limit
concrete	of 2600 kg/m ³
offshore concrete	fixed and floating structures where reinforced concrete, prestressed concrete and
structure	cementitious grout are used as structural materials.
offshore standard	a BKI Rules, Guidelines, Guidances that contain technical requirements, principles and
	acceptance criteria related to certification and classification of offshore units
offshore installation	general term for mobile and fixed structures and facilities which are intended for
	exploration, drilling, production, processing or storage of hydrocarbons including
	installations intended for accommodation of personnel engaged in these activities The
	term covers subsea installations and pipelines but not traditional shuttle tankers,
	supply boats and other support vessels which are not directly engaged in the activities
	described above.
one-compartment	characteristic of a floating object which remains stable even if one of its
damage stability	compartments is flooded
operating	conditions wherein a unit is on location for purposes of production, drilling or other
conditions	similar operations, and combined environmental and operational loadings are within
	the appropriate design limits established for such operations (including normal,
	survival and accidental)
partial load factor	is part of the safety approach and varies in magnitude for the different load categories
n a nua a n - n +	dependent on the individual uncertainties in the characteristic loads
permanent	self-weight, ballast weight, weight of permanent installed parts of mechanical
functional	outfitting, external hydrostatic pressure, prestressing force, etc.
loads (G) post-tensioned	reinforcement (normally tendons, wires, strands or bars) placed inside ducts and
reinforcement	tensioned after the concrete has hardened
potential	voltage between a submerged metal surface and a reference electrode
pre-blended grout	grout proportioned at a factory following strict QA/QC procedures and delivered to
pre sieriaca grout	site for mixing with a predefined proportion of water May refer to pre-packed and silo
	stored/transported products.
prestressing	tendons (wires, strands, and bars), anchorage devices, couplers and ducts or sheaths
systems	are part of a prestressing system May refer to pre-tensioned or post-tensioned
,	systems.

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Table 1.3 Definitions of terms (Continued)

Terms	Definitions
pre-tensioned	reinforcement (normally wires or strands) tensioned before concrete has been placed
reinforcement	
product certificate	certificate to document conformity with the requirements of the applicable standard.
	It lists material properties documented through testing Test samples shall be taken
	from the delivered products themselves and testing, or a part there-of shall be
	performed in the presence of a third party or in accordance with special agreements.
product data sheet	sheet issued by the manufacturer with data about the product which may contain
product data sneet	design data for the product and may be appended to product or type approval
	certificate
quality plan	plan implemented to ensure quality in the design, construction and in-service
quality plan	
	inspection/maintenance
reinforcement	constituents of structural concrete providing the tensile strength that will give the
	reinforced concrete its ductile characteristics, in this guideline, reinforcement is
	categorised as:
	—ordinary reinforcement
	—prestressing reinforcement
	—fibre reinforced polymer reinforcement (limited to carbon, glass, aramid and
	basalt)
	—special reinforcement.
robustness	a robust structure is a structure with low sensitivity to local changes in geometry and
	loads
redundancy	the ability of a component or system to maintain or restore its function when a failure
	of a member or connection has occurred, for instance by introducing alternative load
	paths or force redistribution
reference electrode	electrode with stable open-circuit potential used as reference for potential
	measurements
reliability	the ability of a component or a system to perform its required function without failure
,	during a specified time interval
repair materials	materials used to structurally repair the offshore concrete structure
risk	qualitative or quantitative likelihood of an accidental or unplanned event occurring
TISK	considered in conjunction with the potential consequences of such a failure
	In quantitative terms, risk is the quantified probability of a defined failure mode times
	its quantified consequence.
risk based	decision making technique for inspection planning based on risk – comprising the
inspection	probability of failure and consequence of failure
service temperature	reference temperature on various structural parts of the unit used as a criterion for
. 116	the selection of steel grades or acceptable crack width, etc. in SLS
service life	expected lifetime, or the expected period of use in service of the facility or structure
serviceability limit	limit state corresponding to the criteria applicable to normal use or durability
states (SLS)	
sheaths	ducts for post-tensioning tendons, typically taken to be of semi rigid or rigid type,
	water tight and with adequate stiffness to prevent damages and deformations
short term tensile	the strength of a FRP bar characterized in a standard test in terms of the rupture
strength	strength due to tension that increases at a constant rate till rupture, typically over 1 –
	5 minutes
slamming	impact load on a member from a rising water surface as a wave passes
	May also occur within tanks due to stored liquids.
sloshing	effects caused by the movement of liquid inside a container, which is typically also
	undergoing motion
S-N curve	is a plot of the magnitude of an alternating stress versus the number of cycles to
2 55. 75	failure for a given material
specified minimum	the minimum yield strength prescribed by the specification or standard under which
yield strength	the material is purchased
	the material is purchased
(SMYS)	

Table 1.3 Definitions of terms (Continued)

Terms	Definitions
specified value	minimum or maximum value during the period considered which may take into
opcomed value	account operational requirements, limitations and measures taken such that the
	required safety level is obtained
splash zone	external surfaces of the unit that are periodically exposed to water, the determination
5p.45255	of which includes evaluation of waves, tidal variations, settlements, subsidence and
	vertical motions
stability	ability of the floating structure to remain upright and floating when exposed to small
Stability	changes in applied loads The ability of a structural member to carry small additional
	loads without buckling.
standards	technical requirements and acceptance criteria.
stanuarus	In the context of this document, the term standard shall be understood to cover
	document types such as codes, guidelines and recommended practices in addition to
	bona fide standards.
structural concrete	cementitious composite material which is the main ingredient for construction of
structural concrete	
	concrete structures
structural grout	a cementitious material which is part of the load carrying system of the structure with
	a characteristic compressive strength higher than 35 MPa containing cement, water
	and often additions, admixtures and appropriate fine aggregates
	Structural grout may be cement grout, pre-blended grout and fibre reinforced grout.
submerged zone	part of the unit which is below the splash zone, including buried parts
survival condition	condition during which a unit may be subjected to the most severe environmental
	loadings for
	which the unit is designed when operations such as drilling may have been
	discontinued due
	to the severity of the environmental loadings
	The unit may be either afloat or supported on the sea bed, as applicable.
target safety level	nominal acceptable probability of structural failure
temporary phase	design conditions not covered by operating conditions, e.g. conditions during
conditions	fabrication, mating and installation phases, transit and towing phases, accidental conditions
test report	document made by the manufacturer which contains the results of control tests on
test report	current production, carried out on products having the same method of manufacture
	as the consignment, but not necessarily from the delivered products themselves
tensile strength	for steel it is the minimum stress level where strain hardening is at maximum or at
tensile strength	rupture For concrete it is the direct tensile strength
tovt	
text	tow size in grams per km length of tow or fibre
time to rupture	time it takes from when a specified load is applied until this load causes rupture of the
	FRP bar, may refer to both fatigue and stress rupture normally, the time to rupture
	under a constant sustained load is measured
tow	untwisted bundle of fibres in the form they are delivered on bobbins by the fibre
1 11 111	supplier (synonym: roving, untwisted yarn)
transit conditions	unit movements from one geographical location to another
type approval	BKI certificate documenting approval of conformity with specified requirements on
certificate (TAC)	the basis of a systematic examination of one or more specimens of a product
	representative for the production
ultimate limit states	limit state corresponding to the maximum load carrying resistance
(ULS)	
unit	general term for an offshore structure
utilization factor	fraction of anode material that may be utilised for design purposes
variable functional	weight and loads caused by the normal operation of the offshore structure which may
loads (Q)	vary in position, magnitude and direction during the operational period and includes;
	modules, gas weight, stored goods, pressure of stored components, pressures from
	stored LNG, temperature of LNG, loads occurring during installation, operational boat
	impacts, mooring loads etc.
	stored LNG, temperature of LNG, loads occurring during installation, operational bo

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Table 1.3 Definitions of terms (Continued)

Terms	Definitions
verification	confirmation, through the provision of objective evidence (analysis, observation, measurement, test, records or other evidence), that specified requirements have been fulfilled (ISO 9000:2015)
yarn	twisted bundle of fibres, twisted tow
works certificate	document signed by the manufacturer stating conformity with BKI rule requirements, that tests are carried on samples taken from the delivered product itself and that tests are witnessed and signed by a qualified department of the manufacturer

2. Abbreviations

Abbreviations as shown in Table 1.4 are used in this guideline.

Table 1.4 Abbreviations

Abbreviations	In full
А	Accidental loads
ACI	American Concrete Institute
AISC	American Institute of Steel Construction
ALE	abnormal level earthquake
ALS	Accidental limit states
API	American Petroleum Institute
ASR	Alkali silica reaction
ASTM	American Society for Testing and Materials
BS	British Standard (issued by British Standard Institute)
CoG	Centre of gravity
D	Deformation loads
DDF	Deep draught floaters
E	Environmental loads
ELE	extreme level earthquake
EN	European norm
ETM	Event tree method
ESD	Emergency shut down
FLS	Fatigue limit state
FM	Fracture mechanics
FMEA	Failure mode effect analysis
FRP	fibre reinforced polymer
FTM	Fault tree method
G	Permanent loads
HAT	Highest astronomical tide
HAZOP	Hazop and operability study
HISC	Hydrogen induced stress cracking
HPC	high performance concrete
HS	High strength
IGC	International gas carrier
IMO	International maritime organisation
ISO	International organisation of standardisation
LAT	Lowest astronomic tide
LNG	Liquified natural gas
LRFD	Load and resistance factor design
LWA	Light weight aggregate concrete
LWAC	lightweight aggregate concrete
MPI	Magnetic particle inspection
MSA	manufacturing survey arrangement
MSF	Module support frame

Table 1.4 Abbreviations (Continued)

Abbreviations	In full
MSL	Mean sea level
NACE	National Association of Corrosion Engineers
NDT	Non-destructive testing
NS	Norwegian standard
NW	Normal weight concrete
OPC	ordinary Portland cement
QRA	Quantitative risk analysis
RP	Recommended practice
SLS	Serviceability limit state
SMYS	specified minimum yield stress
TAC	type approval certificate
TTR	time to rupture
ULS	ultimate limit state
UR	utilization ratio
W/C	Water to cement ratio in a mix, may include pozzolanic or latent hydraulic additions

3. Symbols - Greek characters

3.1 Symbols, Greek characters, as shown in Table 1.5 are used in this guideline.

Table 1.5 Greek characters

α	angle between transverse shear reinforcement and the longitudinal axis
	also
	angle between the reinforcement and the contact surface, where only reinforcement with an angle
	between 90° and 45° (to the direction of the force) shall be taken into account.
α_c	factor applicable to $E_{\it cn}$ and $f_{\it cn}$ for offshore concrete structures Section 6, C.1.11
$\alpha_{\scriptscriptstyle f}$	thermal expansion coefficient of FRP reinforcement
$\alpha_{\scriptscriptstyle t}$	factor applicable to $f_{\it in}$ for offshore concrete structures. Section 6, C.1.11
β	opening angle of the bend Section 6, L.1.12
δ	deflection
Δ_{σ}	stress variation of the reinforcement (MPa) Section 6, M.2.2
ε	strain
\mathcal{E}_1	average principal tensile strain Section 6, H.1.7
\mathcal{E}_{cu}	max strain, NW concrete (2,5 m $-$ 1,5) \mathcal{E}_{cn} Section 6,C.1.14
\mathcal{E}_{cm}	mean stress dependent tensile strain in the concrete at the same layer and over the same length as
Cm	\mathcal{E}_{sm} Section 6, 0.8.2
\mathcal{E}_{cs}	free shrinkage strain of the concrete (negative value) Section 6, O.8.2
\mathcal{E}_{s1}	tensile strain in reinforcement slightly sensitive to corrosion on the side with highest strain Section
\$1	6, O.3.7
\mathcal{E}_{s2}	tensile strain at the level of the reinforcement sensitive to corrosion Section 6, O.3.7
\mathcal{E}_{sm}	mean principal tensile strain in the reinforcement in the crack's influence length at the outer layer
- sm	of the reinforcement Section 6, O.8.2
γ_c	material coefficient concrete
γ_f	partial load factor
γ_m	material factor (material coefficient)
γ_s	material factor for steel reinforcement

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Table 1.5 Greek characters (Continued)

$ \begin{array}{ll} \gamma_F & material factor to account for statistical variation in the material strength, potential placement inaccuracy in the section due to the physical characteristics of the bars and the level of control implemented during manufacturing of FRP bars $		
$\begin{array}{ll} \gamma_{II} & \text{material factor to be used for ULS check with load combination type I for FRP bars} \\ \gamma_{IR} & \text{material factor to be used for ULS check with load combination type II for FRP bars} \\ \gamma_{FR} & \text{material factor to be used for ULS check with load combination type III for FRP bars} \\ \gamma_{F.x.v.} & \text{material factor to be used for lous term safe service life assessment for FRP bars} \\ \gamma_{F.x.v.} & \text{material factor to be used in accidental limit states for FRP bars} \\ \gamma_{F.E} & \text{material factor to be used in accidental limit states for FRP bars} \\ \gamma_{F.E} & \text{material factor to be used in accidental limit states for FRP bars} \\ \ell & \text{material factor to be used in accidental limit states for FRP bars} \\ \ell & \text{geometric slenderness ratio} \\ \ell_{N} & \text{force dependent slenderness} \\ \theta & \text{geometric slenderness ratio} \\ \theta & \text{geometric slenderness ratio} \\ \theta & \text{diameter of the reinforcement bar} \\ \phi_{e} & \text{equivalent diameter in term of reinforcement cross-section} \\ \mu & \text{friction coefficient} \\ \rho & \text{coefficient of Findley's creep rate equation} \\ \rho_{P} & \text{density} & \text{density} \\ \rho_{R} & \text{matrix density} \\ \rho_{S} & \text{reinforcement ratio in } x - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } y - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } y - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } \gamma - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } \gamma - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } \gamma - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } \gamma - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } \gamma - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } \gamma - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } \gamma - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } \gamma - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } \gamma - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{S} & \text{reinforcement ratio in } \gamma - $	γ_F	inaccuracy in the section due to the physical characteristics of the bars and the level of control
$\begin{array}{ll} \gamma_{FIII} & \text{material factor to be used for ULS check with load combination type II for FRP bars} \\ \gamma_{FIIII} & \text{material factor to be used for ULS check with load combination type III for FRP bars} \\ \gamma_{FL,shirt} & \text{material factor to be used for long term safe service life assessment for FRP bars} \\ \gamma_{FL} & \text{material factor to be used in accidental limit states for FRP bars} \\ \gamma_{FL} & \text{material factor applied to Young's modulus to account for long term creep of the FRP bars} \\ \gamma_{FL} & \text{material factor to be used in serviceability limit states for FRP bars} \\ \gamma_{FL} & \text{material factor to be used in serviceability limit states for FRP bars} \\ \ell & \text{geometric slenderness ratio} \\ \ell_{\gamma} & \text{force dependent slenderness} \\ \theta & \text{angle between the inclined concrete compression struts and the longitudinal axis in the truss model method} \\ \phi & \text{diameter of the reinforcement bar} \\ \phi_{\ell} & \text{equivalent diameter in term of reinforcement cross-section} \\ \mu & \text{friction coefficient} \\ \rho_{\ell} & \text{coefficient of Findley's creep rate equation} \\ \rho_{\ell} & \text{density of FRP bars} \\ \rho_{f} & \text{fibre density} \\ \rho_{r} & \text{reinforcement ratio in } \mathbf{x} - \text{direction} = A_{xx}/(\mathbf{b} \cdot \mathbf{d}) \\ \eta_{\ell} & \text{inimit for cumulative damage ratio} \\ \eta_{\ell} & \text{conversion factor for bends for the bend radiuses covered} \\ \eta_{f,T,TR} & conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H experature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H experature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore$	$\gamma_{_{FI}}$	
$\begin{array}{ll} \pmb{\gamma}_{FIM} & \text{material factor to be used for ULS check with load combination type III for FRP bars} \\ \pmb{\gamma}_{F,sna} & \text{material factor to be used for long term safe service life assessment for FRP bars} \\ \pmb{\gamma}_{FA} & \text{material factor to be used in accidental limit states for FRP bars} \\ \pmb{\gamma}_{FE} & \text{material factor applied to Young's modulus to account for long term creep of the FRP bars} \\ \text{It is used to determine strains and deformations for ULS, SLS, FLS and ALS.} \\ \pmb{\gamma}_{FS} & \text{material factor to be used in serviceability limit states for FRP bars} \\ \ell & \text{geometric slenderness ratio} \\ \ell_{N} & \text{force dependent slenderness} \\ \theta & \text{angle between the inclined concrete compression struts and the longitudinal axis in the truss model method} \\ \phi & \text{diameter of the reinforcement bar} \\ \phi & \text{diameter of the reinforcement bar} \\ \phi & \text{equivalent diameter in term of reinforcement cross-section} \\ \mu & \text{friction coefficient} \\ \rho & \text{coefficient of Findley's creep rate equation} \\ \rho_{F} & \text{density of FRP bars} \\ \rho_{f} & \text{fibre density} \\ \rho_{R} & \text{reinforcement ratio in x - direction = $A_{xx}/(\text{b} \cdot \text{d})$} \\ \rho_{y} & \text{reinforcement ratio in y - direction = $A_{xx}/(\text{b} \cdot \text{d})$} \\ \eta_{f,TTR} & \text{conversion factor for bends for the bend radiuses covered} \\ \eta_{f,TTR} & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H ϕ concrete stress due to bending alone (tension positive) Section 6, 0.8.1 ϕ_{M} design stress ϕ_{M} edge stress due to bending alone (tension positive) Section 6, 0.8.1 ϕ_{M} numerically largest compressive stress, calculated as the average value within each stress-block ϕ_{M} numerically largest compressive stress, calculated as the average value within each stress-block ϕ_{M} numerically least compressive stress, c$		material factor to be used for ULS check with load combination type II for FRP bars
$\begin{array}{ll} y_{F,x,x,y} & \text{material factor to be used for long term safe service life assessment for FRP bars} \\ y_{FR} & \text{material factor to be used in accidental limit states for FRP bars} \\ y_{FE} & material factor applied to Young's modulus to account for long term creep of the FRP bars lit is used to determine strains and deformations for ULS, SLS, FLS and ALS. $		material factor to be used for ULS check with load combination type III for FRP bars
$\begin{array}{ll} y_{FA} & \text{material factor to be used in accidental limit states for FRP bars} \\ y_{FE} & \text{material factor applied to Young's modulus to account for long term creep of the FRP bars it is used to determine strains and deformations for ULS, SLS, FLS and ALS. \\ y_{FS} & \text{material factor to be used in serviceability limit states for FRP bars} \\ \ell & \text{geometric slenderness ratio} \\ \ell_{N} & \text{force dependent slenderness} \\ \theta & \text{angle between the inclined concrete compression struts and the longitudinal axis in the truss model method} \\ \phi & \text{diameter of the reinforcement bar} \\ \phi_{c} & \text{equivalent diameter in term of reinforcement cross-section} \\ \mu & \text{friction coefficient} \\ \rho & \text{coefficient of Findley's creep rate equation} \\ \rho_{F} & \text{density of FRP bars} \\ \rho_{f} & \text{fibre density} \\ \rho_{m} & \text{matrix density} \\ \rho_{x} & \text{reinforcement ratio in } x - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_{y} & \text{reinforcement ratio in } y - \text{direction = } A_{xx}/(b \cdot d) \\ \eta & \text{limit for cumulative damage ratio} \\ \eta_{f,TTR} & \text{conversion factor for bends for the bend radiuses covered} \\ \eta_{f,TTR} & \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \eta_{t_{imp}} & conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H $		material factor to be used for long term safe service life assessment for FRP bars
$\begin{array}{ll} \gamma_{FE} & \text{material factor applied to Young's modulus to account for long term creep of the FRP bars it is used to determine strains and deformations for ULS, SLS, FLS and ALS. \\ \gamma_{FS} & \text{material factor to be used in serviceability limit states for FRP bars} \\ \ell & \text{geometric slenderness ratio} \\ \ell_N & \text{force dependent slenderness} \\ \theta & \text{angle between the inclined concrete compression struts and the longitudinal axis in the truss model method} \\ \phi & \text{diameter of the reinforcement bar} \\ \phi_e & \text{equivalent diameter in term of reinforcement cross-section} \\ \mu & \text{friction coefficient} \\ \rho & \text{coefficient of Findley's creep rate equation} \\ \rho & \text{density of FRP bars} \\ \rho_F & \text{density of FRP bars} \\ \rho_F & \text{fibre density} \\ \rho_{m} & \text{matrix density} \\ \rho_{\gamma} & \text{reinforcement ratio in } x-\text{direction = } A_{xx}/(b \cdot d) \\ \eta & \text{limit for cumulative damage ratio} \\ \eta_{f,TTR} & \text{conversion factor for bends for the bend radiuses covered} \\ \eta_{f,TTR} & conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H \text{$		material factor to be used in accidental limit states for FRP bars
$\begin{array}{ll} \gamma_{FS} & \text{material factor to be used in serviceability limit states for FRP bars} \\ \ell & \text{geometric slenderness ratio} \\ \ell_N & \text{force dependent slenderness} \\ \theta & \text{angle between the inclined concrete compression struts and the longitudinal axis in the truss model method} \\ \phi & \text{diameter of the reinforcement bar} \\ \phi_e & \text{equivalent diameter in term of reinforcement cross-section} \\ \mu & \text{friction coefficient} \\ \rho & \text{coefficient of Findley's creep rate equation} \\ \rho & \text{density} \\ \rho_F & \text{density of FRP bars} \\ \rho_f & \text{fibre density} \\ \rho_m & \text{matrix density} \\ \rho_w & \text{reinforcement ratio in } x - \text{direction = } A_{xx}/(b \cdot d) \\ \rho_y & \text{reinforcement ratio in } y - \text{direction = } A_{xy}/(b \cdot d) \\ \eta & \text{limit for cumulative damage ratio} \\ \eta_{f,TTR} & \text{conversion factor for bends for the bend radiuses covered} \\ \eta_{f,TTR} & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature \\ \eta_{long} & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H \phi & \text{creep coefficient} \\ \sigma_F & \text{stress in a FRP bar in response to specified loading (referred to nominal bar area)} \\ \sigma_d & \text{design stress} \\ \sigma_{man} & \text{numerically largest compressive stress, calculated as the average value within each stress block} \\ \sigma_{min} & \text{numerically largest compressive stress, calculated as the average value within each stress-block} \\ \sigma_{min} & \text{numerically largest compressive stress, calculated as the average value within each stress-block} \\ \sigma_{min} & \text{numerically largest compressive stress, calculated as the average value within each stress-block} \\ \sigma_{min} & \text{numerically largest compressive stress, calculated} \\ \sigma_{min} & numer$		
$\begin{array}{ll} \ell & \text{geometric slenderness ratio} \\ \ell_N & \text{force dependent slenderness} \\ \theta & \text{angle between the inclined concrete compression struts and the longitudinal axis in the truss} \\ \phi & \text{diameter of the reinforcement bar} \\ \phi_c & \text{equivalent diameter in term of reinforcement cross-section} \\ \mathcal{U} & \text{friction coefficient} \\ \mathcal{P} & \text{coefficient of Findley's creep rate equation} \\ \mathcal{P} & \text{density of FRP bars} \\ \mathcal{P}_F & \text{density of FRP bars} \\ \mathcal{P}_g & \text{reinforcement ratio in } x - \text{direction = } A_{xx}/(b \cdot d) \\ \mathcal{P}_y & \text{reinforcement ratio in } y - \text{direction = } A_{xy}/(b \cdot d) \\ \mathcal{P}_y & \text{reinforcement ratio in } y - \text{direction = } A_{xy}/(b \cdot d) \\ \mathcal{P}_g & \text{donversion factor for bends for the bend radiuses covered} \\ \mathcal{\eta}_{f,TTR} & \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \mathcal{\eta}_{T,TTR} & \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \mathcal{\eta}_{T,TTR} & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature} \\ \mathcal{\eta}_{may} & temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H $		
$\begin{array}{ll} \ell_N & \text{force dependent slenderness} \\ \theta & \text{angle between the inclined concrete compression struts and the longitudinal axis in the truss model method} \\ \phi & \text{diameter of the reinforcement bar} \\ \phi_e & \text{equivalent diameter in term of reinforcement cross-section} \\ \mu & \text{friction coefficient} \\ \rho & \text{coefficient of Findley's creep rate equation} \\ \rho & \text{density of FRP bars} \\ \rho_F & \text{density of FRP bars} \\ \rho_f & \text{fibre density} \\ \rho_m & \text{matrix density} \\ \rho_w & \text{reinforcement ratio in } \mathbf{x} - \text{direction = } A_{xx}/(\mathbf{b} \cdot \mathbf{d}) \\ \rho_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction = } A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \eta_b & \text{conversion factor for bends for the bend radiuses covered} \\ \sigma_{p_{T,TTR}} & \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \eta_T & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature} \\ \theta & \text{creep coefficient} \\ \theta_F & \text{stress in a FRP bar in response to specified loading (referred to nominal bar area)} \\ \theta_G & \text{concrete stress due to long-term loading} \\ \theta_G & \text{design stress} \\ \theta_M & \text{edge stress due to bending alone (tension positive) Section 6, 0.8.1} \\ \theta_{max} & \text{numerically largest compressive stress, calculated as the average value within each stress block} \\ \theta_{min} & \text{numerically least compressive stress, calculated as the average value within each stress block} \\ \theta_{min} & \text{numerically least compressive stress, calculated as the average value within each stress block} \\ \theta_{min} & \text{numerically least compressive stress, calculated as the average value within each stress block} \\ \theta_{min} & \text{numerically least compressive stress, calculated as the average value within each stress block} \\ \theta_{min} & \text{numerically least compressive stress, calculated as the average value within each stress block} \\ \theta_{min} & \text{value} & v$		
$\begin{array}{ll} \theta & \text{angle between the inclined concrete compression struts and the longitudinal axis in the truss model method} \\ \phi & \text{diameter of the reinforcement bar} \\ \phi_c & \text{equivalent diameter in term of reinforcement cross-section} \\ \mathcal{H} & \text{friction coefficient} \\ \mathcal{P} & \text{coefficient of Findley's creep rate equation} \\ \mathcal{P} & \text{density of FRP bars} \\ \mathcal{P}_f & \text{density} \\ \mathcal{P}_F & \text{density} \\ \text{matrix density} \\ \mathcal{P}_w & \text{matrix density} \\ \mathcal{P}_w & \text{reinforcement ratio in } \mathbf{x} - \text{direction} = A_{xx}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{conversion factor for bends for the bend radiuses covered} \\ \mathcal{P}_{f,TTR} & \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \mathcal{H}_{f,TTR} & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H \mathcal{P}_f & \text{stress in a FRP bar in response to specified loading (referred to nominal bar area)} \\ \mathcal{F}_f & \text{stress in the fibres in a FRP bar in response to specified loading (referred to net fibre area)} \\ \mathcal{F}_d & \text{concrete stress due to long-term loading} \\ \mathcal{F}_d & \text{design stress} \\ \mathcal{F}_d & \text{design stress} \\ \mathcal{F}_{max} & \text{numerically largest compressive stress, calculated as the average value within each stress block} \\ \mathcal{F}_{max} & \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \mathcal{F}_{max} & \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \mathcal{F}_{max} & numerically least compressive stress, calculated as the avera$	-	
$\begin{array}{c} \textbf{model method} \\ \textbf{\phi} \\ \textbf{diameter of the reinforcement bar} \\ \textbf{\phi}_{e} \\ \textbf{equivalent diameter in term of reinforcement cross-section} \\ \textbf{\mu} \\ \textbf{friction coefficient} \\ \textbf{\rho} \\ \textbf{coefficient of Findley's creep rate equation} \\ \textbf{\rho} \\ \textbf{density} \\ \textbf{\rho}_{F} \\ \textbf{density of FRP bars} \\ \textbf{\rho}_{f} \\ \textbf{ibre density} \\ \textbf{\rho}_{m} \\ \textbf{matrix density} \\ \textbf{\rho}_{x} \\ \textbf{reinforcement ratio in x - direction = $A_{xx}/(\text{b} \cdot \text{d})$} \\ \textbf{\rho}_{y} \\ \textbf{reinforcement ratio in y - direction = $A_{xy}/(\text{b} \cdot \text{d})$} \\ \textbf{\eta} \\ \textbf{limit for cumulative damage ratio} \\ \textbf{\eta}_{b} \\ \textbf{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \textbf{\eta}_{T,TTR} \\ \textbf{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore A_{TNEX} H as the size in a FRP bar in response to specified loading (referred to nominal bar area) \sigma_{f} stress in a FRP bar in response to specified loading (referred to net fibre area) \sigma_{c} concrete stress due to long-term loading \sigma_{d} design stress \sigma_{d} design stress \sigma_{d} edge stress due to bending alone (tension positive) Section 6, 0.8.1 \sigma_{max} numerically largest compressive stress, calculated as the average value within each stress block \sigma_{min} numerically least compressive stress, calculated as the average value within each stress-block$		
$\begin{array}{ll} \boldsymbol{\phi_e} & \text{equivalent diameter in term of reinforcement cross-section} \\ \boldsymbol{\mu} & \text{friction coefficient} \\ \boldsymbol{\rho} & \text{coefficient of Findley's creep rate equation} \\ \boldsymbol{\rho} & \text{density} \\ \boldsymbol{\rho_F} & \text{density of FRP bars} \\ \boldsymbol{\rho_f} & \text{fibre density} \\ \boldsymbol{\rho_m} & \text{matrix density} \\ \boldsymbol{\rho_x} & \text{reinforcement ratio in } \mathbf{x} - \text{direction} = \mathbf{A_{xx}}/(\mathbf{b} \cdot \mathbf{d}) \\ \boldsymbol{\eta} & \text{limit for cumulative damage ratio} \\ \boldsymbol{\eta_b} & \text{conversion factor for bends for the bend radiuses covered} \\ \boldsymbol{\eta_{f.TTR}} & \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \boldsymbol{\eta}_{t_{map}} & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature} \\ \boldsymbol{\eta}_{t_{map}} & \text{temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H \\ \boldsymbol{\varphi} & \text{creep coefficient} \\ \boldsymbol{\sigma}_{F} & \text{stress in a FRP bar in response to specified loading (referred to nominal bar area)} \\ \boldsymbol{\sigma}_{c} & \text{concrete stress due to long-term loading} \\ \boldsymbol{\sigma}_{d} & \text{design stress} \\ \boldsymbol{\sigma}_{d} & \text{design stress} \\ \boldsymbol{\sigma}_{max} & \text{numerically largest compressive stress, calculated as the average value within each stress block} \\ \boldsymbol{\sigma}_{min} & \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \end{array}$	θ	
$\begin{array}{ll} \mathcal{H} & \text{friction coefficient} \\ \mathcal{P} & \text{coefficient of Findley's creep rate equation} \\ \mathcal{P} & \text{density} \\ \mathcal{P}_F & \text{density of FRP bars} \\ \mathcal{P}_f & \text{fibre density} \\ \mathcal{P}_m & \text{matrix density} \\ \mathcal{P}_x & \text{reinforcement ratio in } \mathbf{x} - \text{direction} = A_{xx}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{reinforcement ratio in } \mathbf{y} - \text{direction} = A_{xy}/(\mathbf{b} \cdot \mathbf{d}) \\ \mathcal{P}_y & \text{conversion factor for bends for the bend radiuses covered} \\ \mathcal{P}_y & conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature solutions growty concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H $	ϕ	diameter of the reinforcement bar
$\begin{array}{ll} \rho & \operatorname{coefficient of Findley's creep rate equation} \\ \rho & \operatorname{density} \\ \rho_F & \operatorname{density of FRP bars} \\ \\ \rho_f & \operatorname{fibre density} \\ \\ \rho_m & \operatorname{matrix density} \\ \\ \rho_x & \operatorname{reinforcement ratio in x - direction} = A_{sx}/(b \cdot d) \\ \\ \rho_y & \operatorname{reinforcement ratio in y - direction} = A_{sy}/(b \cdot d) \\ \\ \eta & \operatorname{limit for cumulative damage ratio} \\ \\ \eta_b & \operatorname{conversion factor for bends for the bend radiuses covered} \\ \\ \eta_{f,TTR} & \operatorname{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \\ \eta_T & \operatorname{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature} \\ \\ \eta_{t_{cmp}} & temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H $	ϕ_e	equivalent diameter in term of reinforcement cross-section
$\begin{array}{ll} \rho & \text{density of FRP bars} \\ \rho_F & \text{density of FRP bars} \\ \\ \rho_f & \text{fibre density} \\ \\ \rho_m & \text{matrix density} \\ \\ \rho_x & \text{reinforcement ratio in x - direction = $A_{sx}/(\text{b} \cdot \text{d})$} \\ \\ \rho_y & \text{reinforcement ratio in y - direction = $A_{sy}/(\text{b} \cdot \text{d})$} \\ \\ \eta & \text{limit for cumulative damage ratio} \\ \\ \eta_b & \text{conversion factor for bends for the bend radiuses covered} \\ \\ \eta_{f.TTR} & \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \\ \eta_T & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature} \\ \\ \eta_{l_{map}} & \text{temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore $Annex H$ \\ \\ \varphi & \text{creep coefficient} \\ \\ \sigma_F & \text{stress in a FRP bar in response to specified loading (referred to nominal bar area)} \\ \\ \sigma_c & \text{concrete stress due to long-term loading} \\ \\ \sigma_d & \text{design stress} \\ \\ \sigma_M & \text{edge stress due to bending alone (tension positive) Section 6, O.8.1} \\ \\ \sigma_{\text{max}} & \text{numerically largest compressive stress, calculated as the average value within each stress-block} \\ \\ \hline \end{array}$	μ	friction coefficient
$\begin{array}{ll} \rho_F & \text{density of FRP bars} \\ \rho_f & \text{fibre density} \\ \\ \rho_m & \text{matrix density} \\ \\ \rho_x & \text{reinforcement ratio in x - direction = $A_{xx}/(\text{b} \cdot \text{d})$} \\ \\ \rho_y & \text{reinforcement ratio in y - direction = $A_{xy}/(\text{b} \cdot \text{d})$} \\ \\ \eta & \text{limit for cumulative damage ratio} \\ \\ \eta_b & \text{conversion factor for bends for the bend radiuses covered} \\ \\ \eta_{f,TTR} & \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \\ \eta_T & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature} \\ \text{temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore A_{nnex} H$ $		coefficient of Findley's creep rate equation
$\begin{array}{ll} \rho_f & \text{fibre density} \\ \rho_m & \text{matrix density} \\ \\ \rho_x & \text{reinforcement ratio in x - direction = $A_{sx}/(\text{b} \cdot \text{d})$} \\ \\ \rho_y & \text{reinforcement ratio in y - direction = $A_{sy}/(\text{b} \cdot \text{d})$} \\ \\ \eta & \text{limit for cumulative damage ratio} \\ \\ \eta_b & \text{conversion factor for bends for the bend radiuses covered} \\ \\ \eta_{f,TTR} & \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \\ \eta_T & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature} \\ \text{temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore $Annex$ H$} \\ \varphi & \text{creep coefficient} \\ \sigma_F & \text{stress in a FRP bar in response to specified loading (referred to nominal bar area)} \\ \sigma_c & \text{concrete stress due to long-term loading} \\ \sigma_d & \text{design stress} \\ \sigma_M & \text{edge stress due to bending alone (tension positive) Section 6, 0.8.1} \\ \sigma_{\text{min}} & \text{numerically largest compressive stress, calculated as the average value within each stress-block} \\ \hline \\ \sigma_{\text{min}} & \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \hline \end{array}$	ρ	
$\begin{array}{ll} \rho_m & \text{matrix density} \\ \rho_x & \text{reinforcement ratio in x - direction} = A_{sx}/(\text{b} \cdot \text{d}) \\ \rho_y & \text{reinforcement ratio in y - direction} = A_{sy}/(\text{b} \cdot \text{d}) \\ \eta & \text{limit for cumulative damage ratio} \\ \eta_b & \text{conversion factor for bends for the bend radiuses covered} \\ \eta_{f,TTR} & \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \eta_T & \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature} \\ temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H \varphi & \text{creep coefficient} \\ \sigma_F & \text{stress in a FRP bar in response to specified loading (referred to nominal bar area)} \\ \sigma_g & \text{concrete stress due to long-term loading} \\ \sigma_d & \text{design stress} \\ \sigma_M & \text{edge stress due to bending alone (tension positive) Section 6, O.8.1} \\ \sigma_{\text{max}} & \text{numerically largest compressive stress, calculated as the average value within each stress-block} \\ \sigma_{\text{min}} & \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \end{array}$	$ ho_{\scriptscriptstyle F}$	density of FRP bars
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$ ho_f$	fibre density
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$ ho_{\scriptscriptstyle m}$	matrix density
$ \eta_b \qquad \text{limit for cumulative damage ratio} \\ \eta_b \qquad \text{conversion factor for bends for the bend radiuses covered} \\ \eta_{f,TTR} \qquad \text{conversion factor derived from the characteristic time to rupture curve for the load durations under consideration} \\ \eta_T \qquad \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature} \\ \eta_{t_{emp}} \qquad \text{temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H creep coefficient \sigma_F \qquad \text{stress in a FRP bar in response to specified loading (referred to nominal bar area)} \\ \sigma_f \qquad \text{stress in the fibres in a FRP bar in response to specified loading (referred to net fibre area)} \\ \sigma_c \qquad \text{concrete stress due to long-term loading} \\ \sigma_d \qquad \text{design stress} \\ \sigma_M \qquad \text{edge stress due to bending alone (tension positive) Section 6, O.8.1} \\ \sigma_{\text{max}} \qquad \text{numerically largest compressive stress, calculated as the average value within each stress block} \\ \sigma_{\text{min}} \qquad \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \sigma_{\text{min}} \qquad \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \sigma_{\text{min}} \qquad \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \sigma_{\text{min}} \qquad \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \sigma_{\text{min}} \qquad \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \sigma_{\text{min}} \qquad \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \sigma_{\text{min}} \qquad \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \sigma_{\text{min}} \qquad \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \sigma_{\text{min}} \qquad numerically least compre$	$\rho_{\scriptscriptstyle x}$	reinforcement ratio in x – direction = A_{sx} /(b · d)
$ \eta_b \qquad \text{conversion factor for bends for the bend radiuses covered} $	$ ho_{ m y}$	reinforcement ratio in y – direction = A_{sy} /(b · d)
$\eta_{f,TTR}$ conversion factor derived from the characteristic time to rupture curve for the load durations under consideration η_{T} conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature $\eta_{t_{emp}}$ temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H φ creep coefficient σ_{F} stress in a FRP bar in response to specified loading (referred to nominal bar area) σ_{f} stress in the fibres in a FRP bar in response to specified loading (referred to net fibre area) σ_{c} concrete stress due to long-term loading σ_{d} design stress σ_{M} edge stress due to bending alone (tension positive) Section 6, O.8.1 σ_{max} numerically largest compressive stress, calculated as the average value within each stress block σ_{min} numerically least compressive stress, calculated as the average value within each stress-block	η	
$\eta_T \qquad \text{under consideration} \\ \eta_T \qquad \text{conversion factor for tensile strength of FRP reinforcement from room temperature to specified service temperature} \\ \eta_{t_{emp}} \qquad \text{temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H \varphi \qquad \text{creep coefficient} \\ \sigma_F \qquad \text{stress in a FRP bar in response to specified loading (referred to nominal bar area)} \\ \sigma_f \qquad \text{stress in the fibres in a FRP bar in response to specified loading (referred to net fibre area)} \\ \sigma_c \qquad \text{concrete stress due to long-term loading} \\ \sigma_d \qquad \text{design stress} \\ \sigma_M \qquad \text{edge stress due to bending alone (tension positive) Section 6, O.8.1} \\ \sigma_{\text{max}} \qquad \text{numerically largest compressive stress, calculated as the average value within each stress block} \\ \sigma_{\text{min}} \qquad \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \hline \end{tabular} $	η_{b}	conversion factor for bends for the bend radiuses covered
$\eta_{t_{emp}} \qquad \text{temperature} \\ \eta_{t_{emp}} \qquad \text{temperature constant to allow for inaccuracies in maintaining and recording low temperatures during grout/ concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H \varphi \qquad \text{creep coefficient} \\ \sigma_F \qquad \text{stress in a FRP bar in response to specified loading (referred to nominal bar area)} \\ \sigma_f \qquad \text{stress in the fibres in a FRP bar in response to specified loading (referred to net fibre area)} \\ \sigma_c \qquad \text{concrete stress due to long-term loading} \\ \sigma_d \qquad \text{design stress} \\ \sigma_M \qquad \text{edge stress due to bending alone (tension positive) Section 6, O.8.1} \\ \sigma_{\text{max}} \qquad \text{numerically largest compressive stress, calculated as the average value within each stress block} \\ \sigma_{\text{min}} \qquad \text{numerically least compressive stress, calculated as the average value within each stress-block} \\ \hline$	$\eta_{_{f,TTR}}$	
$\sigma_{F} = \frac{\text{during grout/concrete testing as well as inaccuracies associated with temperature forecasting offshore Annex H}{\sigma_{F}} = \frac{\sigma_{F}}{\sigma_{F}} = \sigma$	$\eta_{\scriptscriptstyle T}$	· · · · · · · · · · · · · · · · · · ·
$arphi_F$ creep coefficient σ_F stress in a FRP bar in response to specified loading (referred to nominal bar area) σ_f stress in the fibres in a FRP bar in response to specified loading (referred to net fibre area) σ_c concrete stress due to long-term loading σ_d design stress σ_M edge stress due to bending alone (tension positive) Section 6, O.8.1 σ_{\max} numerically largest compressive stress, calculated as the average value within each stress block σ_{\min} numerically least compressive stress, calculated as the average value within each stress-block	$\eta_{t_{emp}}$	during grout/ concrete testing as well as inaccuracies associated with temperature forecasting
σ_f stress in the fibres in a FRP bar in response to specified loading (referred to net fibre area) σ_c concrete stress due to long-term loading σ_d design stress σ_M edge stress due to bending alone (tension positive) Section 6, O.8.1 σ_{max} numerically largest compressive stress, calculated as the average value within each stress block σ_{min} numerically least compressive stress, calculated as the average value within each stress-block	φ	
σ_c concrete stress due to long-term loading σ_d design stress σ_d design stress edge stress due to bending alone (tension positive) Section 6, 0.8.1 σ_{max} numerically largest compressive stress, calculated as the average value within each stress block σ_{min} numerically least compressive stress, calculated as the average value within each stress-block	$\sigma_{\scriptscriptstyle F}$	stress in a FRP bar in response to specified loading (referred to nominal bar area)
σ_d design stress σ_d edge stress due to bending alone (tension positive) Section 6, 0.8.1 σ_{max} numerically largest compressive stress, calculated as the average value within each stress block σ_{min} numerically least compressive stress, calculated as the average value within each stress-block	$\sigma_{\scriptscriptstyle f}$	stress in the fibres in a FRP bar in response to specified loading (referred to net fibre area)
σ_{M} edge stress due to bending alone (tension positive) Section 6, O.8.1 σ_{max} numerically largest compressive stress, calculated as the average value within each stress block σ_{min} numerically least compressive stress, calculated as the average value within each stress-block	σ_c	concrete stress due to long-term loading
$\sigma_{ m max}$ numerically largest compressive stress, calculated as the average value within each stress block numerically least compressive stress, calculated as the average value within each stress-block	$\sigma_{\scriptscriptstyle d}$	design stress
$\sigma_{ m max}$ numerically largest compressive stress, calculated as the average value within each stress block numerically least compressive stress, calculated as the average value within each stress-block	$\sigma_{\scriptscriptstyle M}$	edge stress due to bending alone (tension positive) Section 6, O.8.1
min in the state of the state o		numerically largest compressive stress, calculated as the average value within each stress block
		numerically least compressive stress, calculated as the average value within each stress-block
		stress due to axial force (tension positive) Section 6,0.8.1

Table 1.5 Greek characters (Continued)

$\sigma_{\scriptscriptstyle P}$	steel stress due to prestressing
$\sigma_{\scriptscriptstyle trough}$	stress at the trough of the stress cycle (minimum stress)
$\sigma_{_{peak}}$	peak stress of the stress cycle (maximum stress)
$ au_{cd}$	bond strength
$ au_{b ext{max}}$	maximum bond stress within fatigue stress block
$ au_{b ext{min}}$	minimum bond stress within fatigue stress block
v_f	volume fraction of fibre in FRP bar

4. Symbols - Latin characters

Symbols, Latin characters, as shown in Table 1.6 are used in this guideline.

Table 1.6 Latin characters

Symbol	Description
A	distance from the face of the support
A_1	loaded area
A_2	assumed distribution area
A_c	concrete area of a longitudinal section of the beam web
A_c	cross-sectional area of uncracked concrete
A_{cf}	effective cross section area of the flange, $h_{_{\! f}}b_{_{\! e\!f\!f}}$
A_{F}	cross-sectional area of FRP reinforcement
A_f	Net fibre area in a FRP reinforcement bar
$A_{F,BAR}$	cross-sectional area of each FRP reinforcement bar
$A_{F, \min}$	minimum area of FRP reinforcement needed to prevent excessive cracking
$A_{F,tow}$	net fibre area of tow
$A_{F,V}$	amount of FRP shear reinforcement with spacing $ m{s} $ [mm 2]
$A_{F,v\mathrm{min}}$	minimum amount of FRP shear reinforcement with spacing $oldsymbol{s}$ [mm 2]
A_{Fs}	nominal FRP bar surface area
A_s	cross-sectional area of steel reinforcement or Reinforcement area that is sufficiently anchored on both sides of the joint and that is not utilized for other purposes
A_{st}	area of transverse reinforcement not utilized for other tensile forces and having spacing not greater than 12 times the diameter of the anchored reinforcement. If the reinforcement is partly utilized, the area shall be proportionally reduced
A_{sv}	amount of shear reinforcement
A_{sx}	amount of reinforcement in x-direction
A_{sy}	amount of reinforcement in y-direction
a_{v}	vertical acceleration
$b_{\it eff}$	part of the slab width which according to Section 6, A.4 is assumed as effective when resisting tensile forces
$b_{\scriptscriptstyle w}$	width of beam (web) [mm]
b_{x}	length of the side of the critical section Section 6, F.5.10

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Table 1.6 Latin characters (Continued)

Symbol	Description
b_{y}	length of the side perpendicular to $oldsymbol{b}_{_{\scriptscriptstyle X}}$
С	coefficient of characteristic safe service life formula for FRP bar specification
C	concrete grade (normal weight concrete)
C_1	factor on Wöhler curves concrete Section 6, M.2
C_1	minimum concrete cover, see Table 6.15
C_2	factor on Wöhler curves concrete section 6, M.2
C_2	actual nominal concrete cover
C_3	factor on Wöhler curve reinforcement Section 6, M.2
C_4	factor on Wöhler curve reinforcement Section 6, M.2
C_5	fatigue strength parameter Section 6,M.2
D	deformation load
D	distance from the centroid of the tensile reinforcement to outer edge of the compression zone
d_1	1000 mm
	nominal diameter of FRP bar
D_F	
D_k	diameter of the concrete core inside the centroid of the spiral reinforcement, A_{ss} environmental load
$\frac{E}{e}$	eccentricity of loading
E_{cd}	design value of Young's Modulus of concrete used in the stress-strain curve
E_{cn}	normalized value of Young's Modulus of concrete used in the stress-strain curve
	characteristic value of the Young's modulus of FRP reinforcement bar (referred to nominal bar
E_F	area $A_{\scriptscriptstyle F}$)
E_{Fd}	design value of Young's Modulus of FRP bars
E_{sd}	design value of Young's Modulus of steel reinforcement
E_{sk}	characteristic value of Young's Modulus of steel reinforcement (200000 MPa)
f_{bc}	concrete related portion of the design bond strength in accordance with Section 6, K.1.16
f_{bd}	design bond strength, calculated in accordance with Section 6, K.1.16
f_{c2d}	truss analogy: design compressive strength Section 6, F.3.8 in the compression field General: reduced design compressive strength Section 6, G.1.7
f_{cck}	characteristic compressive cylinder strength of concrete or grout
f_{cck2}	94 MPa Section 4, C.3.7
f_{cckj}	characteristic strength of the drilled cores converted into cylinder strength for cylinders with height/diameter ratio 2:1
f_{cckt}	characteristic compressive cylinder strength at 28 days based on in-situ tests
F_{cd}	compressive capacity
f_{cd}	design compressive strength of concrete/grout
f_{ck}	characteristic concrete/grout cube strength
f_{cn}	normalized compressive strength of concrete/grout
F_d	design load

Sec 1 General F

Table 1.6 Latin characters (Continued)

	Description
$F_{\scriptscriptstyle F}$	tensile force at rupture of FRP bar
$f_{\scriptscriptstyle F}$	characteristic short term tensile strength (force per area) of FRP bar
$f_{F,bend}$	characteristic tensile strength of bent portion of FRP bar
f_{Fb}	design strength of the bend portion of FRP bar
$f_{\it Fd}$	Design strength of FRP reinforcement
$f_{F,TTR(i)}$	characteristic tensile strength (force per area) in FRP bar until failure at considered load duration, i, derived from characteristic TTR curve. i is taken as I, II, III corresponding to load durations of 50 years, 1 year, and 1 week respectively
F_{k}	characteristic load
F_{rd}	reference strength for use in fatigue calculation, dependant on the type of failure in question Section 6, M.2
$F_{rd,fat}$	reference strength for use in fatigue calculation, dependant on the type of failure in question
	Section 6, M.2 including the material specific factor C_5
f_{sd}	design strength of steel reinforcement
f_{sk}	characteristic strength of steel reinforcement
f_{ssd}	design strength of the spiral reinforcement, $oldsymbol{A}_{ss}$
F_{SV}	additional tensile force in longitudinal reinforcement due to shear
f_{td}	design strength of concrete/grout in uniaxial tension
f_{tk}	characteristic uniaxial tensile strength of concrete/grout
f_{tk}	f_{tk} $+$ $0.5P_{w}$ for structures exposed to pressure from liquid or gas in the formulae for
	calculating the required amount of minimum reinforcement Section 6, Q.6.3
f_{tn}	normalized tensile strength of concrete/grout
F_{VN}	force corresponding to shear failure at cross wire welds within the development length
G	permanent load
g,g_o	acceleration due to gravity
Н	cross-section height
$H^{'}$	distance between the centroid of the reinforcement on the tensile" and compression side of the member
h_f	thickness of the flange (the slab)
l_c	moment of inertia of A_c
L	length of FRP bar
$l_b^{'}$	development length for welded wire fabric
l_b	development length bond – bars or bundle of bars
l_{bp}	development length for the prestressing force
l_e	effective length, theoretical buckling length
L_i	distance between zero moment points
l_{sk}	influence length of the crack considering that some slippage in the bond between reinforcement and concrete may occur Section 6, C.8.2
М	$arepsilon_{co}/arepsilon_{cn}$
m	total moment in the section acting in combination with the shear force $oldsymbol{V}_f$

Sec 1 General F

Table 1.6 Latin characters (Continued)

Symbol	Description
m_f	mass fraction of fibres (average from production records)
$m_{_m}$	average mass fraction of matrix resin $\left(m_{_{\! m}}=1-m_{_{\! f}} ight)$
$ M_{OA} $	numerical smallest member end moment calculated from 1. order theory at end A
$ M_{OB} $	numerical largest member end moment calculated from 1. order theory at end B
m_{tex}	tow or fibre mass expressed in tex [g/km]
N	exponent of Findley's creep rate equation
N	design life of concrete subjected to cyclic stresses
n_f	$N_f/f_{cd}A_c$
N_f	design axial force (positive as tension)
n_i	number of cycles in stress-block i Section 6, M.1.8
N_{i}	number of cycles with constant amplitude which causes fatigue failure Section 6, M.1.8
N_{x}	axial force in x-direction
N_{xy}	shear force in the x-y plane
N_y	axial force in y-direction
P	load
P	pressure
P_d	design pressure
Q	variable functional load
R	radius
r_c	radius of curvature
R_d	design resistance
R_k	characteristic resistance
S	centre to centre distance between the spiral reinforcement, measured in the longitudinal direction of the
	column Section 6, D.1.6 or, spacing between shear reinforcement in longitudinal direction
S_1	spacing of the transverse reinforcement
S_c	area moment about the centroid axis of the cross-section for one part of the concrete section
S_d	design load effect
S_k	characteristic load effect
T	specified longitudinal tolerance for the position of the bar end
$t_{app,\max}$	maximum temperature of application, defined by the manufacturer, for a grout or fibre reinforced grout
	Shall be taken as +30°C in the absence of data from an elevated temperature test programme.
$t_{app, \min}$	minimum temperature of application, defined by the manufacturer, for a grout or fibre reinforced grout
	Shall be taken as +5°C in the absence of data from a low temperature test programme.
$t_{test, \max}$	temperature which the equipment, constituent materials and test and curing environments shall be maintained at during material testing of grout to be qualified for application at temperatures above 30°C

Table 1.6 Latin characters (Continued)

Symbol	Description
$t_{test, \min}$	temperature which the equipment, constituent materials and test and curing environments shall be
	maintained at during material testing of grout to be qualified for application at temperatures below +5°C
V_{ccd}	design shear capacity of a concrete cross-section (shear compression mode of failure)
V_{cd}	design shear capacity of a concrete cross-section (shear tension made of failure)
V_f	design shear force for the cross-section under consideration
$V_{ m max}$	maximum shear force within fatigue stress block
$V_{ m min}$	minimum shear force within fatigue stress block
V_{sd}	design shear capacity of transverse reinforcement (shear tension mode of failure)
W_c	section modulus of the concrete cross-section with respect to the extreme tension fibre or the fibre with least compression
w_k	nominal characteristic crack widths
Z	0,9 d for sections with a compression zone
Z_1	the greater of 0,7 d and $\left.I_{c}\middle/S_{c}\right.$

5. Verbal forms

5.1 Verbal forms, as shown in Table 1.7 are used in this guideline.

Table 1.7 Definitions of verbal forms

Term	Definition
shall	verbal form used to indicate requirements strictly to be followed in order to conform to the
	document
should	verbal form used to indicate that among several possibilities one is recommended as particularly suitable, without mentioning or excluding others, or that a certain course of action is preferred but not necessarily required
may	verbal form used to indicate a course of action permissible within the limits of the document

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